



ADDENDUM 1

September 1, 2020

ITB TDD 71-20

Destin Fort Walton Beach Convention Center Exterior Accessibility

This addendum is to extend bid opening, add additional bid documents, and answer questions provided by the vendors.

This addendum incorporates:

- Project Manual: Add Southern Earth Sciences: March 25, 2020, File No.: P20-0128, Geotechnical Investigation and recommendations.
- Prebid Conference Sign In Sheets: August 19, 2020.
- Sheet ED101: Add Note #5 "Remove Step Lights (3). Remove existing conductors and cap existing conduit."
- Existing Entry Conduit Photos

Questions and Responses:

Q1. Sheet E102 is on the Index of Drawings on Sheet G000, but is not included in the bid package?

R1. Sheet E102 does not exist. Void Sheet G000 reference.

Q2. Electrical drawings have general notes and keynotes. There are no electrical specifications in the project manual. Are there going to be any Electrical specification issued for this project?

R2. All electrical specifications are provided on the drawing sheets.

Q3. Section 084213 – Paragraph 2.9 states to match existing glass. Please provide additional information on the type of glass. It is impossible to look at a piece of glass and know actually which coatings and glass type that was used?

R3.

XG Impact Resistant Rated Glass to be:

- Exterior Pane: Solarban® 70XL on Optigray® 6mm – Heat Strengthened
- Interlayer: 0.090 inch thick PVB complying with large missile impact testing and wind speed Requirement of Florida Building Code.
- Interior Pane: 6mm Clear – Heat Strengthened.

TG Tempered Glass to be:

- Nominal ¼" thick fully tempered clear float glass.

Q4. Section 084213 – Paragraph 2.8.A - Previous project that have bid at the Convention Center required Electric panics, Continuous hinges, Pulls, and LCN Closers. Are these items of hardware required for the doors that are a part of this project?

R4.

Door 002B

- 2 - Continuous Hinge: A110HDC 95 CUT TO CUSTOM LENGTH M TW-8 AB
- 2 - Exit Device 9600AA 8FT ALD C CD J MLR MS 630 DM
- 2 - Exit Device Trim 9BPO03 J 626 DM
- 4 - Cormax Core 1CM-7MJ12 626 BE
- 2 - Offset Pull 1191-3 630 TR
- 2 - Auto Operators Reuse Existing 2 - Door Position Switch MC4 DM
- 2 - Power Supply PS615RF UR-2A DM
- 2 - Push Plate Actuator Reuse Existing BY
- 1 - Meeting Stile Seal By Alum Storefront Mfg. BY
- 1 - Weatherstrip By Alum Dr. Mfg. BY
- 1 - Lip Threshold 896 P AL NA

Doors 002A, 003A, 003B

- 2 - Continuous Hinge: A110HDC 95 CUT TO CUSTOM LENGTH M TW-8 AB
- 2 - Exit Device 9600AA 8FT ALD C CD J MLR MS 630 DM
- 2 - Exit Device Trim 9BPO03 J 626 DM
- 4 - Cormax Core 1CM-7MJ12 626 BE
- 2 - Offset Pull 1191-3 630 TR
- 1 - Meeting Stile Seal By Alum Storefront Mfg. BY
- 1 - Weatherstrip By Alum Dr. Mfg. BY
- 1 - Lip Threshold 896 P AL NA

Q5. Grid Shift Solutions – Solar Lighting Substitution Request

R5. Rejected based on:

- Poles must meet EPA Rating of 150mph with Fixtures and solar panel as specified. Fixture sheet has no specifics to show equal to specified. (Sheet is just a brochure not an actual specification sheet).

Q6. On page ED101 note 4 phase 7 it calls for the existing backboard to be repaired, but due to the condition I think it needs to be totally replaced because we will be installing new electrical equipment if phase 7 is accepted. Also, if phase 7 is accepted and the engineer decides to replace the backboard, we will need a detail on what will be required?

R6. Repair/demolish existing backboard, provide new PT 6X6 posts and PT planking as required. Include a \$1,200 allowance.

Q7. On page E101 phase 7 shows us installing a new underground service from the existing backboard to the new pool pump panel. What it does not say is what the engineer intends to do for as doing a directional bore or doing a cut, remove and replace the asphalt, pavers, and concrete etc.?

R7. Either directional bore or cut and patch will be acceptable. Contractor to choose best cost option.

Q8. For the meter and breaker enclosures that is to be installed on the backboard it does not say if they want standard 3R enclosures or stainless steel. Due to the environment stainless will last longer being it is so close to the water.

R8. 3R Stainless Steel.

Q9. Can we substitute Star Mag for Sweet Bay due to salt spray and soil type?

R9. Use Red Bay (Persea Borbonia) in lieu for the Sweet Bay.

Q10. Can we substitute Trailing Rosemary 3 gallon for 1 gallon due to product availability?

R10. Yes.

Q11. They don't tell us anything about a well other than include it, "if needed". We don't even know if we can get decent water out of a well that close to the gulf. I recommend using potable water since the landscape area is so small. Please advise.

R11. Use existing water wells

Q12. There are some existing concrete steps to the left of the entrance as you walk up that are to remain. Do these get pavers? Do we use bull nose coping on the tread? Do we use pavers for the risers?

R12. Steps to the left of the entrance will remain as existing and do not receive pavers. Patch and repair deck coating as required to match existing. See Note on 1/S101. All new exterior stairs shall receive a bull nose coping on the tread and pavers on the risers. See Specification Section 321400-2.1B. New Garage Stair landings, treads and risers shall receive deck coating to match

Q13. What is the project budget estimate?

R13. Over \$1,000,000.00.

The opening date for this ITB has been changed to September 16, 2020 at 3:15 PM CST.

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Panama City Beach, FL 32408

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FL Certificate of Authorization #3737

Okaloosa County Purchasing Dept
5479A Old Bethel Rd
Crestview, FL 32536

March 25, 2020
File No.: P20-0128

C/O: DAG Architects, Attn: Mr. Alex Gacic

Subject: Geotechnical Services for the Proposed Additions at the Emerald Coast Convention Center in Fort Walton Beach, Florida (Contract C18-2638-TDD, Task Order No. 6)

Dear Mr. Gacic:

Southern Earth Sciences, Inc., has completed the geotechnical services for the proposed additions at the Emerald Coast Convention Center in Fort Walton Beach, Florida. Our services were performed in general accordance with our proposal P20-0002.01, dated January 2, 2020. This report presents the results of our field and laboratory testing and includes recommendations with regard to the design and construction of the foundations.

FIELD INVESTIGATIVE PROCEDURES:

Prior to our testing, boring locations were marked and Sunshine State One Call of Florida was contacted to locate underground utilities. On March 16, 2020, personnel with our firm traveled to the project site and completed the field testing for the above referenced project. For our geotechnical investigation, six (6) cone soundings were performed to a depth of 25 feet below the existing ground surface. The cone penetrometer is track mounted and rather than sampling and testing at five foot intervals, as normally done with a standard penetration borings, the cone penetrometer is an electronic device that provides continuous evaluation of the soils bearing capacity through point and frictional resistances. The cone penetrometer is hydraulically pushed into the soil with point and frictional resistances obtained continuously on a computer printout. This testing equipment provides an accurate definition of the soil strength characteristics and the changes in stratification. The cone soundings were performed in general accordance with ASTM D5778.

To verify soil conditions encountered within the depth of our cone soundings, direct push borings were performed to depths ranging from approximately 10 to 15 feet below the existing ground surface. The direct push borings were performed with our Geoprobe 6622 and the DT22 soil sampling system. This is a closed-piston sampler, with an inner piston rod and outer drive casing, and is driven to the top of the sampling interval. The inner piston rod is removed and the sampler is driven to collect a soil sample. The soil samples are collected in a clear 5-foot PVC liner and are delivered back to our laboratory for soil classifications and laboratory testing.

Test locations were established in the field by using a 100-foot tape and referencing existing landmarks; therefore, our test locations should be considered approximate. See the attached Figure for our approximate test locations.

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Fort Walton Beach, FL
Page 2**

LABORATORY TESTING PROCEDURES:

Laboratory investigative work consisted of physical examination of samples obtained during the soil test boring operation. Soil samples were visually classified in the laboratory in accordance with the Unified Soil Classification System. Evaluation of these samples, in conjunction with penetration resistances, have been used to estimate soil characteristics.

Natural Moisture: One (1) sample was selected for determination of their natural moisture content. In the laboratory, each sample was weighed, dried, and its moisture content was calculated in general accordance with ASTM D2216.

The laboratory test results are shown on the boring logs at the depth of the tested sample. Abbreviations of laboratory data are shown below:

NM = Natural Moisture Content (%)

CONE SOUNDINGS:

CPT Log graphically indicates the cone tip resistance, friction ratio, equivalent N-value and interpreted soil type at each sounding location. Soil classifications and data were interpreted from methods recommended by Robertson and Campanella and/or the Swedish Geotechnical Institute Information Publication No. 15E. Correlations between Cone Resistance values and Standard Penetration Testing “N” values were performed according to the methods developed by Robertson, Campanella and Wightman.

The soil types and stratigraphy shown on the CPT Log sheets are based upon material parameters measured and evaluated as the cone is advanced. The CPT Log sheets were developed for general information only.

SITE AND SOIL CONDITIONS:

The additions will be on the east and northeast sides of the convention center. These are typically in landscaped or grassed areas. There are sidewalks and asphalt paved areas around the building and landscaped areas. Topographic information is not available, however, based upon our in-field observations the elevation slopes downward away from the main entrance.

The logs of our cone soundings and direct push borings are attached. Topographic information for the site is not available, therefore, the elevations of our test locations are unknown.

The soils encountered within the depth of our cone soundings and direct push borings were predominantly sands. The sands varied in color and texture, which ranged from slightly

Emerald Coast Convention Center Additions
Fort Walton Beach, FL
Page 3

silty to clean sands. Thin layers of organic laden sands (peaty sands) were encountered at test locations C-1, C-2, and C-3 at approximate depths of 14 feet and 9 feet, respectively. The peaty sands are very loose to loose, however, were approximately 2 to 4-inches thick, based upon the results of our borings. Generally, the sands within the top two to three feet are loose and become medium dense to an average depth of approximately nine feet. Thin sporadic loose layers are encountered between 10 and 15 feet, however the remaining depths of our cone soundings, the sands are medium dense to very dense. At test location C-4 the sands were generally loose within the top ten feet, based upon the results of our cone sounding. Minimal amounts of organics (topsoil) were encountered within the top six inches. It should also be noted, gravel was encountered at C-1 at a depth of approximately 4 feet and at test location C-4 concrete rubble at a depth of 1 foot below existing ground surface.

On the date of our field testing (March 16, 2020), the groundwater level was measured at the depths indicated on the attached logs, which ranged from approximately 2.4 to 9.0 feet below the existing ground surface. Fluctuations in the water table depths will occur due to seasonal precipitation/evapotranspiration differences, changes in topography, wetland / neighboring drainage influences and tidal influences; therefore, prior to foundation construction we recommend that groundwater levels be verified.

STRUCTURAL INFORMATION:

The existing parking garage is supported on a deep (pile) foundation. The proposed additions will include stairways into the convention center and retaining walls. We understand most of the additions will be constructed at existing grade, and no fill soils, other than those behind retaining walls, will be imported. We have received loading information from the Structural Engineer on the project, Mr. Neill O'Connell, P.E. We understand wall loads on the retaining wall will be 4 kips per lineal foot, and the contact pressure near the stairs and slabs will be on the order of 1000psf, or less. We understand the wall footings will be approximately two feet below existing ground surface. We also understand that a portion of the stairs will be supported on the existing garage foundation supported on piling. If any of this information is incorrect we should be notified to provide additional recommendations.

FOUNDATION RECOMMENDATIONS

Our evaluation of foundation conditions has been based on structural information presented in this report and subsurface data obtained during our investigation. In evaluating soundings and direct push borings, we have used correlations that were previously made between penetration resistances and foundation stabilities observed in soil conditions similar to those encountered at your site.

As previously indicated, with the exception of sounding C-4, the soils become medium dense below a depth of two to three feet below existing grade. For new foundations, we recommend an allowable soil contact pressure not to exceed 1500 psf, or less. We have

**Emerald Coast Convention Center Additions
Fort Walton Beach, FL
Page 4**

calculated settlements of less than one inch for wall footings. We recommend wall footings have a minimum width of 18 inches. We also recommend footings have a minimum embedment depth of 18 inches from the bottom of the footings to the outside finished grade.

Sounding C-4 was anomalous in that the sands were loose to a depth of ten feet and may be a result of loosely placed backfill during the original construction. New foundations in this area should be designed for an allowable soil contact pressure not to exceed 1000 psf. Additional compaction recommendations for this area are indicated below.

Prior to foundation construction we recommend the following site and soil preparations.

1. Clear and grub the surface soils within the building perimeter and extend at least five (5) feet beyond the building perimeter to remove all topsoil, organics, organic laden sands, and other deleterious materials. Based upon the results of our borings, these soils typically extend to a depth of 6-inches, however, concrete rubble was encountered at test location C-4 to a depth of approximately 18-inches below the existing ground surface. However, these soils may extend to greater depths than our borings indicate.
2. Once the site has been grubbed, prior to the placement of fill soils, compact the existing soils until a density of 95% of the Modified Proctor (ASTM D-1557) maximum dry density is achieved to a depth of twelve (12) inches below compacted grade. Moisture conditioning of the soils including dewatering may be required to achieve optimum moisture conditions for compaction. We do not recommend a vibratory roller for compaction within 50 feet of existing structures.

At sounding C-4, we recommend compacting the existing soils to a depth equal to the width of the footing. This may require excavating and stockpiling the existing soils, compacting at the bottom of the excavation and replacing and compacting the stockpiled soils to the bottom of footing elevation.

3. Fill soils shall be sands to slightly silty sands containing no more than twelve (12) percent, by dry weight, finer than U.S. No. 200 mesh sieve. Fill shall be placed in thin level lifts not to exceed twelve (12) inches, loose, and compacted to a density of 95% of the Modified Proctor maximum dry density throughout its full depth.
4. Once footings are excavated, compact the soils at the bottom of footings until a density of 95% of the Modified Proctor maximum dry density is achieved to a depth of two (2) feet below the bottom of footings.
5. Laboratory moisture-density relationships (Proctors) and in-place density tests should be performed to verify compliance with the foregoing compaction recommendations. We recommend one density test per 50 lineal feet of wall footing and one density test per 2000 square feet of existing soils and for each foot of fill soils.

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Fort Walton Beach, FL
Page 5

6. To achieve compaction to the depths indicated above, the groundwater level may be encountered. If encountered, dewatering may be required to lower the groundwater level at least one foot below the bottom of the excavations.

Stair Supports on Existing Foundations

You have indicated that a portion of the stairs will be supported on the existing pile foundation of the parking garage. New loads to the existing pile foundations should be limited to 15-20% of the existing pile design capacity. In addition, we do **not** recommend partial pile support and partial soil support on the same stair foundations due to potential differential settlement problems. If this is the intent of the design, we should discuss this issue with the Structural Engineer.

FOUNDATION STEM WALL

We have assumed the foundation stem walls will have a height of about three feet. The lateral earth pressure resultant acting on the stem wall may be calculated using the following formula.

$$P = 1/2 K_0 \gamma H^2$$

where K_0 = coefficient of at-rest earth pressure (Use 0.45)
 γ = effective soil unit weight Use 110 pcf (unsaturated)
H = height of wall retaining soil (in feet)

The backfill soils behind the stem wall shall be the same as fill under the building compacted to a density of 95% of the Modified Proctor (ASTM D-1557) maximum dry density throughout its full depth. Compaction shall be performed using hand compaction equipment only, within three feet of the stem wall.

Laboratory moisture density relationships (Proctors) and in-place density tests shall be performed to verify compliance with the foregoing compaction criteria. We recommend three in-place density tests per foot of backfill behind the stem wall.

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Fort Walton Beach, FL
Page 6**

TESTING:

The effectiveness of the foundation will depend significantly on the proper preparation of the soils, as indicated previously. Therefore, we recommend the owner employ Southern Earth Sciences, Inc., as the testing laboratory to perform construction testing services. If we are not employed to provide construction testing services, Southern Earth Sciences, Inc, can not accept any responsibility for any conditions, which deviate from those described in this geotechnical report. Southern Earth Sciences, Inc., should be invited to the pre-construction conference to discuss the project with all interested parties so that the project may be completed expeditiously and to the intent of our geotechnical report. We would be pleased to review the plans and specifications as they relate to the soil preparation and provide a fee proposal for construction testing.

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Page 7**

GENERAL COMMENTS:

Professional judgments on design criteria are presented in this letter. These are based partly on our evaluations of technical information provided, partly on our understanding of the characteristics of the project being planned, and partly on our general experience with subsurface conditions in the area. We do not guarantee performance of the project in any respect, only that our judgments meet the standard of care of our profession.

This information is exclusively for the use and benefit of the addressee(s) identified on the first page of this report and is not for the use or benefit of, nor may it be relied upon by any other person or entity. The contents of this letter may not be quoted in whole or in part or distributed to any person or entity other than the addressee(s) hereof without, in each case, the advance written consent of the undersigned.


This report has been prepared in order to aid in the evaluation of this property and to assist the architects and engineers in the foundation, pavement, and stormwater design. It is intended for use with regard to the specific project discussed herein, and any substantial changes in the buildings, loads, locations, or assumed (or reported) grades shall be brought to our attention immediately so that we may determine how such changes may effect our conclusions and recommendations. We would appreciate the opportunity to review the plans and specifications for the foundation and floor construction to verify that our conclusions and recommendations are interpreted correctly. Our report does not address environmental issues which may be associated with the subject property.

While the cone soundings and borings performed for this project are representative of subsurface soil conditions at their respective locations and for their respective vertical reaches, local variations of the subsurface materials are anticipated and may be encountered. The boring logs and related information are based on the driller's logs and visual examination of selected samples in the laboratory. Delineation between soil types shown on the boring logs is approximate, and soil descriptions represent our interpretation of subsurface conditions at the designated boring location on the particular date drilled.

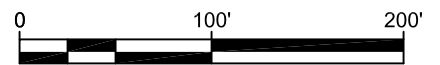
We appreciate the opportunity to assist you. If you have any questions or if we may be of further assistance, please call at your convenience.


Yours Very Truly,

SOUTHERN EARTH SCIENCES, INC.


Logan A. Fowler, P.E.
Eng. Reg. No. 82343
State of Florida





SESI FILE NO: P20-0128	 SOUTHERN EARTH SCIENCES, INC. Geotechnical, Environmental & Construction Materials Testing	DRAWN BY: LF		FIGURE I APPROXIMATE TEST LOCATIONS
Emerald Coast Convention Center Additions Fort Walton Beach, FL		CHECKED BY: LF	DATE: 3/16/20	
		SCALE: 1:100		

Southern Earth Sciences Inc.

ITB TDD 71-20 ADDENDUM 1

Operator: Jamison Short

CPT Date/Time: 3/16/2020 11:12:36 AM

Sounding: C-1

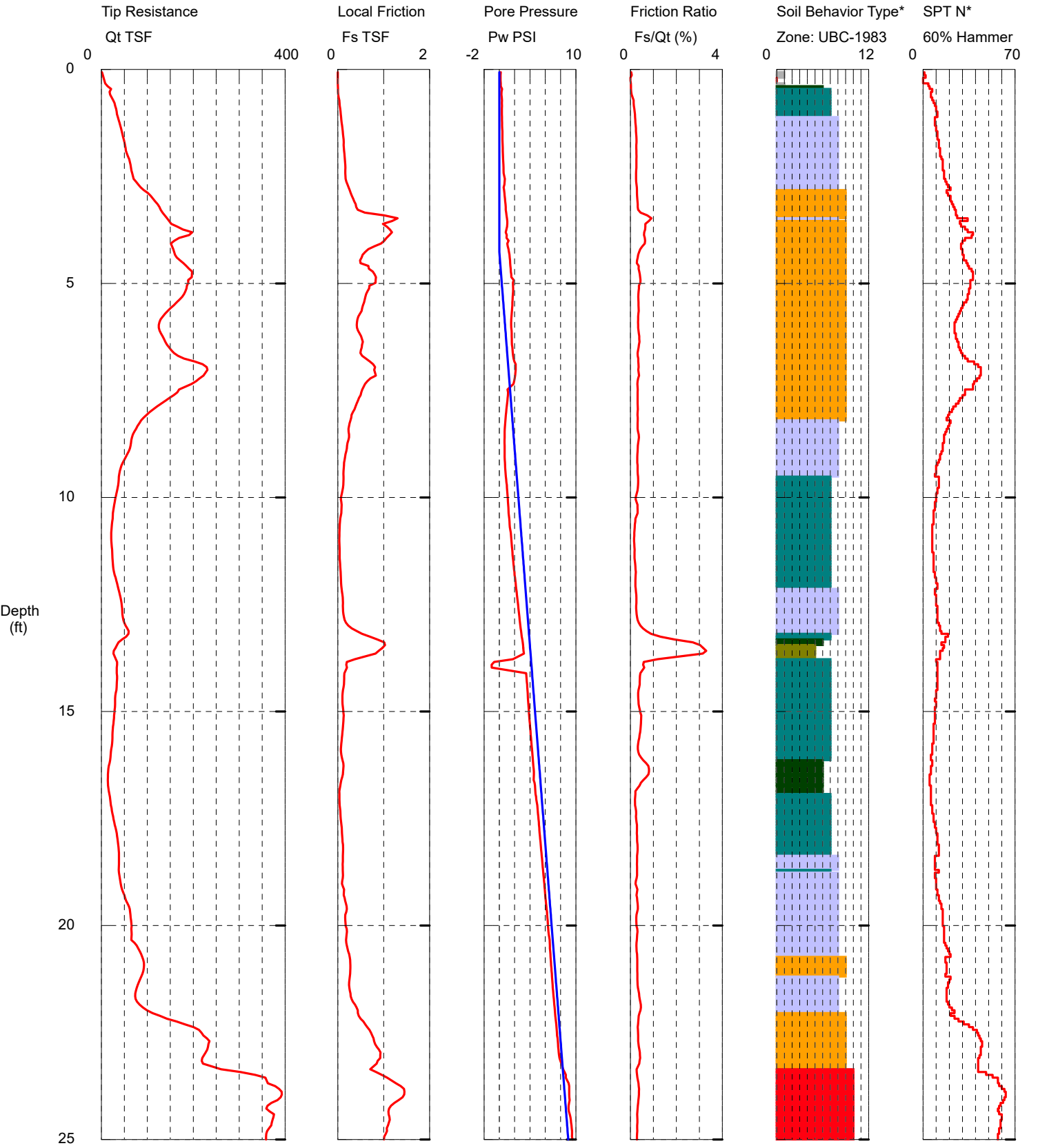
Location: Emerald Coast Convention Center

Cone Used: DDG1485

Job Number: P20-0128

Groundwater Depth: 4.4 ft

Elevation: Unknown



Maximum Depth = 26.12 feet

Depth Increment = 0.066 feet

- | | | | |
|--------------------------|-----------------------------|----------------------------|--------------------------------|
| 1 sensitive fine grained | 4 silty clay to clay | 7 silty sand to sandy silt | 10 gravelly sand to sand |
| 2 organic material | 5 clayey silt to silty clay | 8 sand to silty sand | 11 very stiff fine grained (*) |
| 3 clay | 6 sandy silt to clayey silt | 9 sand | 12 sand to clayey sand (*) |

C-2

C-2

*Soil behavior type and SPT based on data from UBC-1983

LOG OF BORING C-1

PROJECT: Emerald Coast Convention Center Additions
LOCATION: Fort Walton Beach, FL
PROJECT NO.: P20-0128
DATE: 03/16/20

METHOD: Direct Push
DRILLER: JS
ENGR / GEOL: LF
SURFACE ELEVATION: Unknown

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	LOCATION	▲ N Value (blows/ft)				NATURAL MOISTURE (%)	ATTERBERG LIMITS (%)			PASSING #200 SIEVE (%)
			Per Plan	20	40	60	80		LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
			MATERIAL DESCRIPTION	Atterberg Limits Natural Moisture								
0				PL	MC	LL		LL	PL	PI		
		SP	Tan and Light Brown Fine SAND									
		SP	Gravel									
5		SP	Brown and Gray Fine SAND with Gravel									
		SP	Light Gray and Tan Fine SAND									
		SP	Light Gray Fine SAND									
10												
		PT	Dark Gray Peaty Sand with Trace Organics									
15		SP	Dark Gray Fine SAND									
20												

LOG OF BORING P20-0128.GPJ SES PC FL.GDT 3/20/20

Water Level Est. Seasonal High GWL: ▾ Measured: ▾ Perched: ▾ **Notes:**
 Water Observations: Groundwater Measured at 4.4 Feet
 Below Existing Ground Surface

N - SPT Data (Blows/Ft) P - Pocket Penetrometer (tsf)

Sample Key: SPT Shelby Tube

SOUTHERN EARTH SCIENCES, inc.

Southern Earth Sciences Inc.

ITB TDD 71-20 ADDENDUM 1

Operator: Jamison Short

CPT Date/Time: 3/16/2020 11:36:37 AM

Sounding: C-2

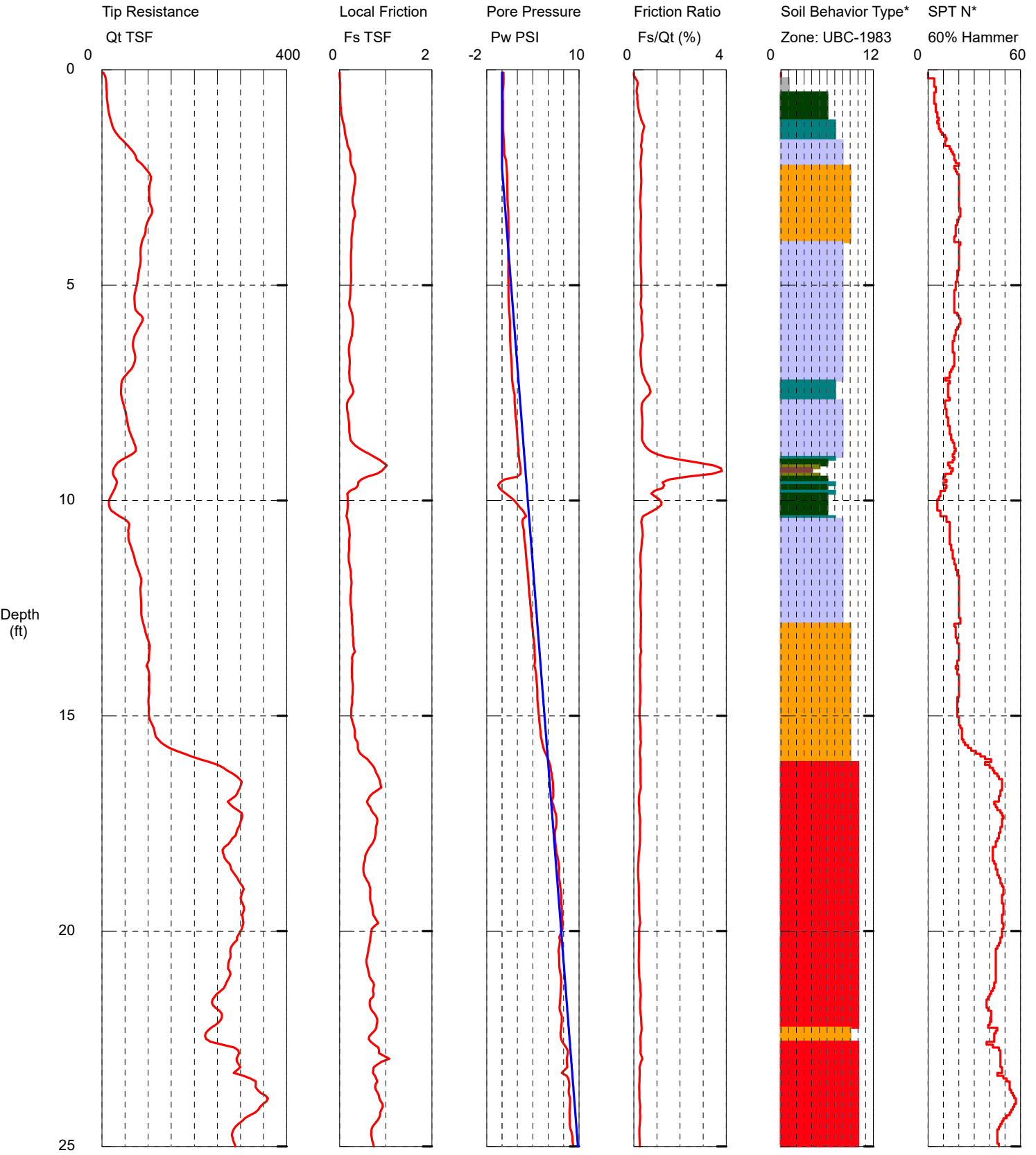
Location: Emerald Coast Convention Center

Cone Used: DDG1485

Job Number: P20-0128

Groundwater Depth: 2.4 ft

Elevation: Unknown



Maximum Depth = 25.66 feet

Depth Increment = 0.066 feet

- | | | | |
|--------------------------|-----------------------------|----------------------------|--------------------------------|
| 1 sensitive fine grained | 4 silty clay to clay | 7 silty sand to sandy silt | 10 gravelly sand to sand |
| 2 organic material | 5 clayey silt to silty clay | 8 sand to silty sand | 11 very stiff fine grained (*) |
| 3 clay | 6 sandy silt to clayey silt | 9 sand | 12 sand to clayey sand (*) |

C-2

*Soil behavior type and SPT based on data from UBC-1983

LOG OF BORING C-2

PROJECT: Emerald Coast Convention Center Additions
LOCATION: Fort Walton Beach, FL
PROJECT NO.: P20-0128
DATE: 03/16/20

METHOD: Direct Push
DRILLER: JS
ENGR / GEOL: LF
SURFACE ELEVATION: Unknown

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	LOCATION	▲ N Value (blows/ft)	NATURAL MOISTURE (%)	ATTERBERG LIMITS (%)			PASSING #200 SIEVE (%)
			Per Plan	20 40 60 80		LIQUID LIMIT LL	PLASTIC LIMIT PL	PLASTICITY INDEX PI	
			MATERIAL DESCRIPTION	Atterberg Limits Natural Moisture					
0		SP	Organics Tan and Light Gray Fine SAND						
5		SP	Light Brown and Light Gray Fine SAND						
10		SP- SM	Gray and Brown Slightly Silty Fine SAND						
10		PT SP- SM	Dark Gray Peaty Sand with Trace Organics Brown and Dark Brown Slightly Silty Fine SAND			84			
15		SP	Gray and Brown Fine SAND						
20									

Water Level Est. Seasonal High GWL: ▾ Measured: ▾ Perched: ▾ **Notes:**
 Water Observations: Groundwater Measured at 2.4 Feet
 Below Existing Ground Surface

N - SPT Data (Blows/Ft) P - Pocket Penetrometer (tsf)

Sample Key: SPT Shelby Tube

SOUTHERN EARTH SCIENCES, inc.

LOG OF BORING P20-0128.GPJ SES PC FL.GDT 3/20/20

Southern Earth Sciences Inc.

ITB TDD 71-20 ADDENDUM 1

Operator: Jamison Short

CPT Date/Time: 3/16/2020 11:57:23 AM

Sounding: C-3

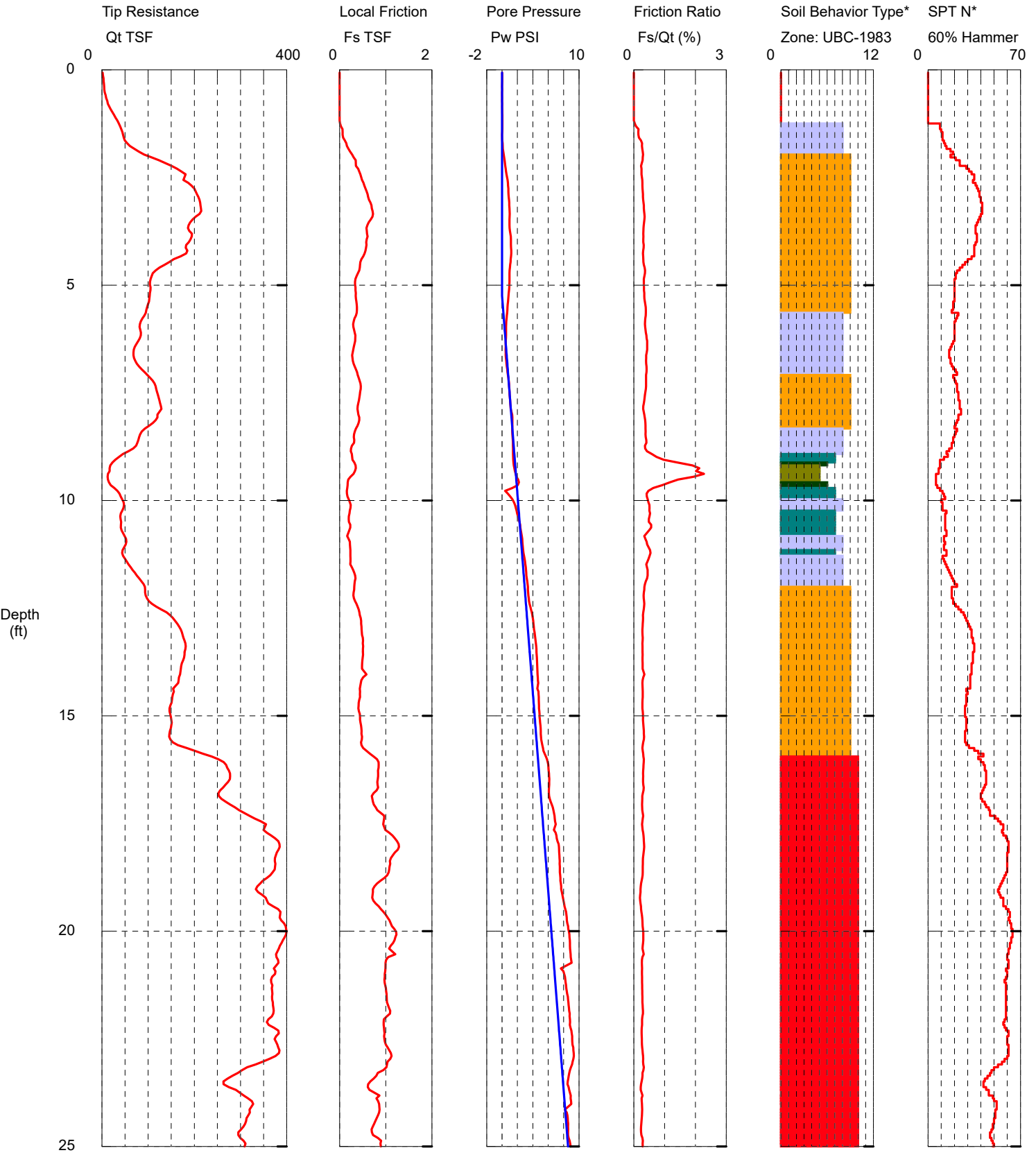
Location: Emerald Coast Convention Center

Cone Used: DDG1485

Job Number: P20-0128

Groundwater Depth: 4.9 ft

Elevation: Unknown



Maximum Depth = 25.13 feet

Depth Increment = 0.066 feet

- | | | | |
|--------------------------|-----------------------------|----------------------------|--------------------------------|
| 1 sensitive fine grained | 4 silty clay to clay | 7 silty sand to sandy silt | 10 gravely sand to sand |
| 2 organic material | 5 clayey silt to silty clay | 8 sand to silty sand | 11 very stiff fine grained (*) |
| 3 clay | 6 sandy silt to clayey silt | 9 sand | 12 sand to clayey sand (*) |

C-2

C-2

*Soil behavior type and SPT based on data from UBC-1983

Southern Earth Sciences Inc.

ITB TDD 71-20 ADDENDUM 1

Operator: Jamison Short

CPT Date/Time: 3/16/2020 10:44:53 AM

Sounding: C-4

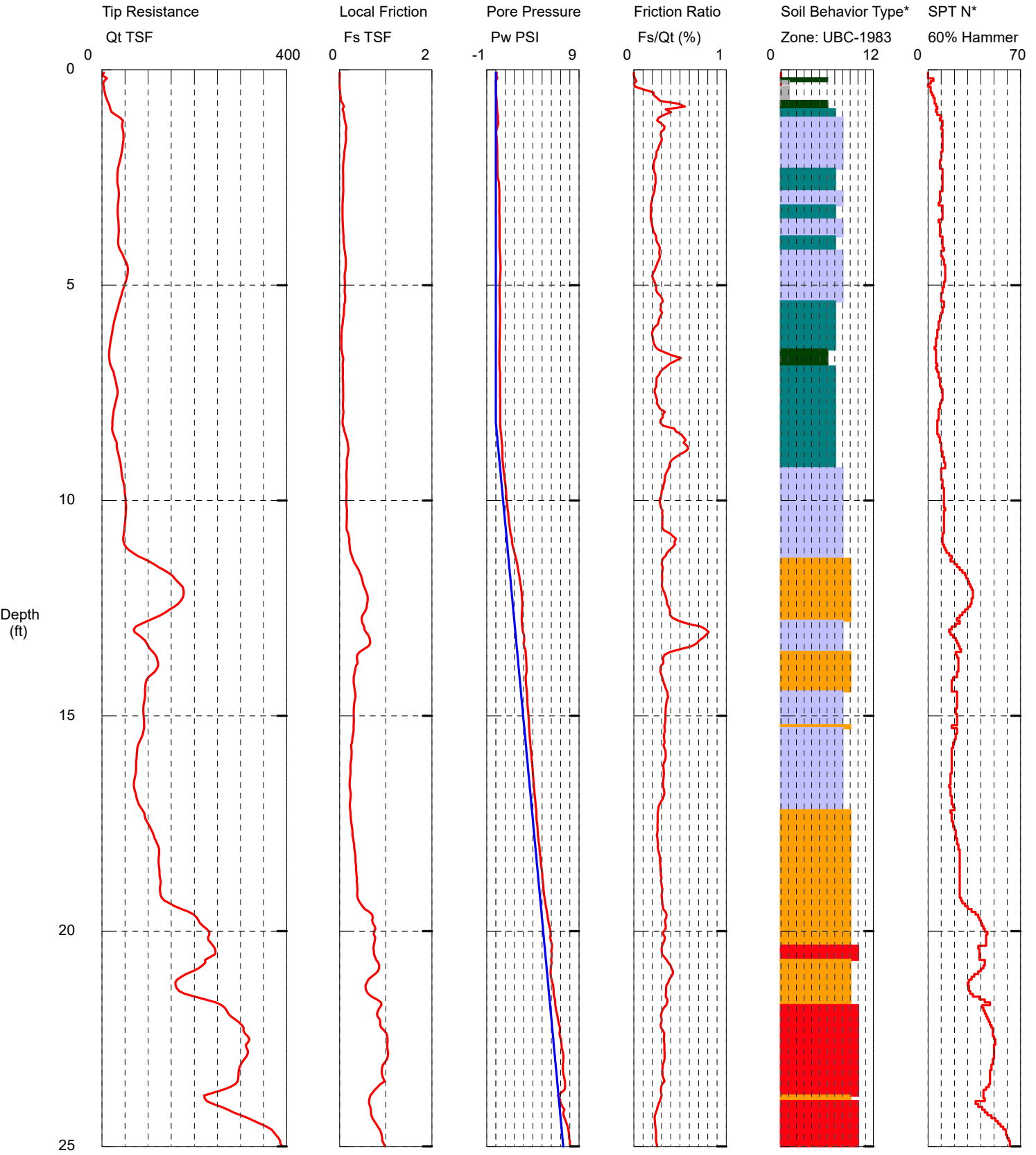
Location: Emerald Coast Convention Center

Cone Used: DDG1485

Job Number: P20-0128

Groundwater Depth: 9.0 ft

Elevation: Unknown



Maximum Depth = 26.12 feet

Depth Increment = 0.066 feet

- | | | | |
|--------------------------|-----------------------------|----------------------------|--------------------------------|
| 1 sensitive fine grained | 4 silty clay to clay | 7 silty sand to sandy silt | 10 gravelly sand to sand |
| 2 organic material | 5 clayey silt to silty clay | 8 sand to silty sand | 11 very stiff fine grained (*) |
| 3 clay | 6 sandy silt to clayey silt | 9 sand | 12 sand to clayey sand (*) |

C-2

Center

*Soil behavior type and SPT based on data from UBC-1983

LOG OF BORING C-4

PROJECT: Emerald Coast Convention Center Additions
LOCATION: Fort Walton Beach, FL
PROJECT NO.: P20-0128
DATE: 03/16/20

METHOD: Direct Push
DRILLER: JS
ENGR / GEOL: LF
SURFACE ELEVATION: Unknown

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	LOCATION	▲ N Value (blows/ft)				NATURAL MOISTURE (%)	ATTERBERG LIMITS (%)			PASSING #200 SIEVE (%)
			Per Plan	20	40	60	80		LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
			Atterberg Limits Natural Moisture				PL					
			MATERIAL DESCRIPTION	20	40	60	80		LL	PL	PI	
0		SP	Tan and Light Gray Fine SAND									
		SP	Concrete Rubble Tan and Light Gray Fine SAND									
5		SP	Light Orange Fine SAND									
		SP	Tan and Light Brown Fine SAND									
10												
15												
20												

Water Level Est. Seasonal High GWL: Measured: Perched: **Notes:**
 Water Observations: Groundwater Measured at 9.0 Feet
 Below Existing Ground Surface

N - SPT Data (Blows/Ft) P - Pocket Penetrometer (tsf)

Sample Key: SPT Shelby Tube

SOUTHERN EARTH SCIENCES, inc.

LOG OF BORING P20-0128.GPJ SES PC FL.GDT 3/20/20

Southern Earth Sciences Inc.

ITB TDD 71-20 ADDENDUM 1

Operator: Jamison Short

CPT Date/Time: 3/16/2020 10:24:21 AM

Sounding: C-5

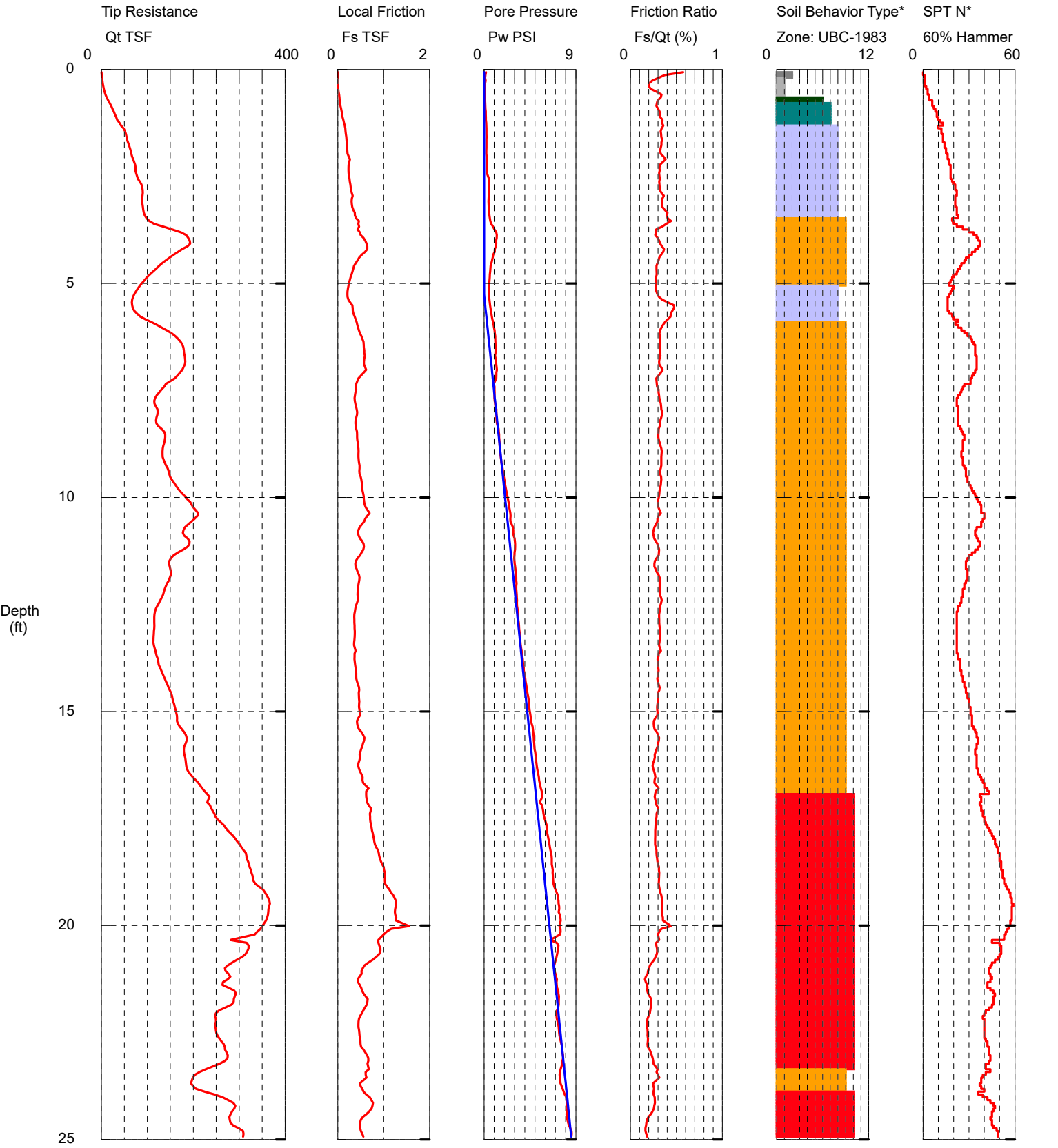
Location: Emerald Coast Convention Center

Cone Used: DDG1485

Job Number: P20-0128

Groundwater Depth: 4.9 ft

Elevation: Unknown



Maximum Depth = 24.93 feet

Depth Increment = 0.066 feet

- | | | | |
|--------------------------|-----------------------------|----------------------------|--------------------------------|
| 1 sensitive fine grained | 4 silty clay to clay | 7 silty sand to sandy silt | 10 gravelly sand to sand |
| 2 organic material | 5 clayey silt to silty clay | 8 sand to silty sand | 11 very stiff fine grained (*) |
| 3 clay | 6 sandy silt to clayey silt | 9 sand | 12 sand to clayey sand (*) |

C-2

C-2

*Soil behavior type and SPT based on data from UBC-1983

Southern Earth Sciences Inc.

ITB TDD 71-20 ADDENDUM 1

Operator: Jamison Short

CPT Date/Time: 3/16/2020 9:59:24 AM

Sounding: C-6

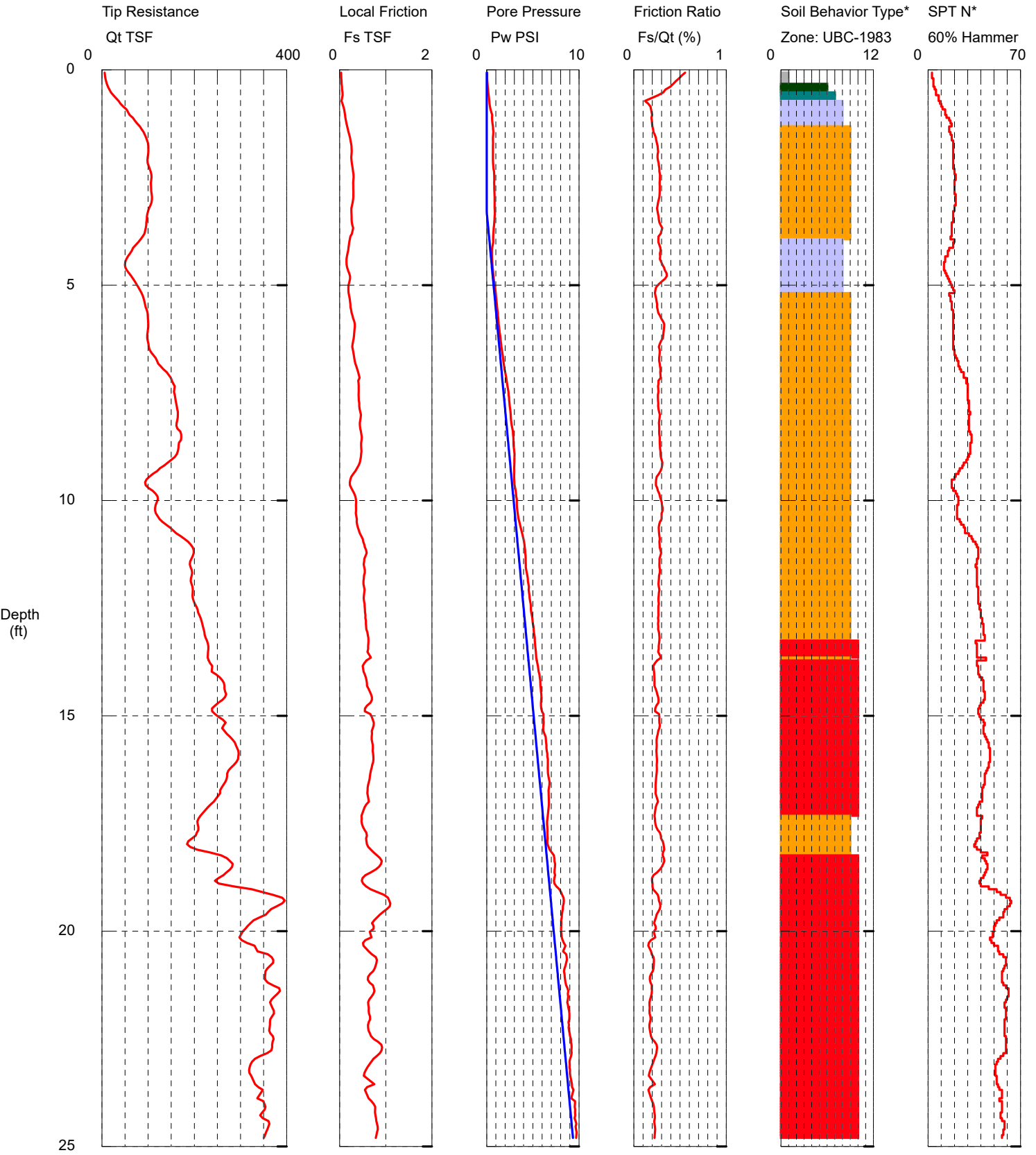
Location: Emerald Coast Convention Center

Cone Used: DDG1485

Job Number: P20-0128

Groundwater Depth: 4.0 ft

Elevation: Unknown



Maximum Depth = 24.80 feet

Depth Increment = 0.066 feet

- | | | | |
|--------------------------|-----------------------------|----------------------------|--------------------------------|
| 1 sensitive fine grained | 4 silty clay to clay | 7 silty sand to sandy silt | 10 gravelly sand to sand |
| 2 organic material | 5 clayey silt to silty clay | 8 sand to silty sand | 11 very stiff fine grained (*) |
| 3 clay | 6 sandy silt to clayey silt | 9 sand | 12 sand to clayey sand (*) |

C-2

C-2

Center

*Soil behavior type and SPT based on data from UBC-1983

LOG OF BORING C-6

PROJECT: Emerald Coast Convention Center Additions
LOCATION: Fort Walton Beach, FL
PROJECT NO.: P20-0128
DATE: 03/16/20

METHOD: Direct Push
DRILLER: JS
ENGR / GEOL: LF
SURFACE ELEVATION: Unknown

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	LOCATION	▲ N Value (blows/ft)	NATURAL MOISTURE (%)	ATTERBERG LIMITS (%)			PASSING #200 SIEVE (%)
			Per Plan	20 40 60 80		Atterberg Limits Natural Moisture			
				PL MC LL					
			MATERIAL DESCRIPTION	20 40 60 80		LL	PL	PI	
0		SP	Tan and Light Gray Fine SAND						
5		SP-SM	Gray Slightly Silty Fine SAND						
10		SP	Gray and Light Gray Fine SAND						
15									
20									

Water Level Est. Seasonal High GWL: Measured: Perched: **Notes:**
 Water Observations: Groundwater Measured at 4.0 Feet
 Below Existing Ground Surface

N - SPT Data (Blows/Ft) P - Pocket Penetrometer (tsf)

Sample Key: SPT Shelby Tube

SOUTHERN EARTH SCIENCES, inc.

LOG OF BORING P20-0128.GPJ SES PC FL GDT 3/20/20

Important Information About Your Geotechnical Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

The following information is provided to help you manage your risks.

Geotechnical Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical engineering study conducted for a civil engineer may not fulfill the needs of a construction contractor or even another civil engineer. Because each geotechnical engineering study is unique, each geotechnical engineering report is unique, prepared *solely* for the client. No one except you should rely on your geotechnical engineering report without first conferring with the geotechnical engineer who prepared it. *And no one — not even you — should apply the report for any purpose or project except the one originally contemplated.*

Read the Full Report

Serious problems have occurred because those relying on a geotechnical engineering report did not read it all. Do not rely on an executive summary. Do not read selected elements only.

A Geotechnical Engineering Report Is Based on A Unique Set of Project-Specific Factors

Geotechnical engineers consider a number of unique, project-specific factors when establishing the scope of a study. Typical factors include: the client's goals, objectives, and risk management preferences; the general nature of the structure involved, its size, and configuration; the location of the structure on the site; and other planned or existing site improvements, such as access roads, parking lots, and underground utilities. Unless the geotechnical engineer who conducted the study specifically indicates otherwise, do not rely on a geotechnical engineering report that was:

- not prepared for you,
- not prepared for your project,
- not prepared for the specific site explored, or
- completed before important project changes were made.

Typical changes that can erode the reliability of an existing geotechnical engineering report include those that affect:

- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light industrial plant to a refrigerated warehouse,

- elevation, configuration, location, orientation, or weight of the proposed structure,
- composition of the design team, or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes—even minor ones—and request an assessment of their impact. *Geotechnical engineers cannot accept responsibility or liability for problems that occur because their reports do not consider developments of which they were not informed.*

Subsurface Conditions Can Change

A geotechnical engineering report is based on conditions that existed at the time the study was performed. *Do not rely on a geotechnical engineering report* whose adequacy may have been affected by: the passage of time; by man-made events, such as construction on or adjacent to the site; or by natural events, such as floods, earthquakes, or groundwater fluctuations. *Always* contact the geotechnical engineer before applying the report to determine if it is still reliable. A minor amount of additional testing or analysis could prevent major problems.

Most Geotechnical Findings Are Professional Opinions

Site exploration identifies subsurface conditions only at those points where subsurface tests are conducted or samples are taken. Geotechnical engineers review field and laboratory data and then apply their professional judgment to render an opinion about subsurface conditions throughout the site. Actual subsurface conditions may differ—sometimes significantly—from those indicated in your report. Retaining the geotechnical engineer who developed your report to provide construction observation is the most effective method of managing the risks associated with unanticipated conditions.

A Report's Recommendations Are *Not* Final

Do not overrely on the construction recommendations included in your report. *Those recommendations are not final*, because geotechnical engineers develop them principally from judgment and opinion. Geotechnical engineers can finalize their recommendations only by observing actual

subsurface conditions revealed during construction. *The geotechnical engineer who developed your report cannot assume responsibility or liability for the report's recommendations if that engineer does not perform construction observation.*

A Geotechnical Engineering Report Is Subject to Misinterpretation

Other design team members' misinterpretation of geotechnical engineering reports has resulted in costly problems. Lower that risk by having your geotechnical engineer confer with appropriate members of the design team after submitting the report. Also retain your geotechnical engineer to review pertinent elements of the design team's plans and specifications. Contractors can also misinterpret a geotechnical engineering report. Reduce that risk by having your geotechnical engineer participate in prebid and preconstruction conferences, and by providing construction observation.

Do Not Redraw the Engineer's Logs

Geotechnical engineers prepare final boring and testing logs based upon their interpretation of field logs and laboratory data. To prevent errors or omissions, the logs included in a geotechnical engineering report should *never* be redrawn for inclusion in architectural or other design drawings. Only photographic or electronic reproduction is acceptable, *but recognize that separating logs from the report can elevate risk.*

Give Contractors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can make contractors liable for unanticipated subsurface conditions by limiting what they provide for bid preparation. To help prevent costly problems, give contractors the complete geotechnical engineering report, *but* preface it with a clearly written letter of transmittal. In that letter, advise contractors that the report was not prepared for purposes of bid development and that the report's accuracy is limited; encourage them to confer with the geotechnical engineer who prepared the report (a modest fee may be required) and/or to conduct additional study to obtain the specific types of information they need or prefer. A prebid conference can also be valuable. *Be sure contractors have sufficient time* to perform additional study. Only then might you be in a position to give contractors the best information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions.

Read Responsibility Provisions Closely

Some clients, design professionals, and contractors do not recognize that geotechnical engineering is far less exact than other engineering disciplines. This lack of understanding has created unrealistic expectations that

have led to disappointments, claims, and disputes. To help reduce the risk of such outcomes, geotechnical engineers commonly include a variety of explanatory provisions in their reports. Sometimes labeled "limitations" many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are Not Covered

The equipment, techniques, and personnel used to perform a *geoenvironmental* study differ significantly from those used to perform a *geotechnical* study. For that reason, a geotechnical engineering report does not usually relate any geoenvironmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated environmental problems have led to numerous project failures.* If you have not yet obtained your own geoenvironmental information, ask your geotechnical consultant for risk management guidance. *Do not rely on an environmental report prepared for someone else.*

Obtain Professional Assistance To Deal with Mold

Diverse strategies can be applied during building design, construction, operation, and maintenance to prevent significant amounts of mold from growing on indoor surfaces. To be effective, all such strategies should be devised for the *express purpose* of mold prevention, integrated into a comprehensive plan, and executed with diligent oversight by a professional mold prevention consultant. Because just a small amount of water or moisture can lead to the development of severe mold infestations, a number of mold prevention strategies focus on keeping building surfaces dry. While groundwater, water infiltration, and similar issues may have been addressed as part of the geotechnical engineering study whose findings are conveyed in this report, the geotechnical engineer in charge of this project is not a mold prevention consultant; *none of the services performed in connection with the geotechnical engineer's study were designed or conducted for the purpose of mold prevention. Proper implementation of the recommendations conveyed in this report will not of itself be sufficient to prevent mold from growing in or on the structure involved.*

Rely on Your ASFE-Member Geotechnical Engineer for Additional Assistance

Membership in ASFE/The Best People on Earth exposes geotechnical engineers to a wide array of risk management techniques that can be of genuine benefit for everyone involved with a construction project. Confer with your ASFE-member geotechnical engineer for more information.



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PRE-BID SIGN IN SHEET
for
OKALOOSA COUNTY TDC
DESTIN FORT WALTON BEACH CONVENTION CENTER
EXTERIOR ACCESSIBILITY

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PRE-BID SIGN IN SHEET
for
OKALOOSA COUNTY TDC
DESTIN FORT WALTON BEACH CONVENTION CENTER
EXTERIOR ACCESSIBILITY

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PRE-BID SIGN IN SHEET
for
OKALOOSA COUNTY TDC
DESTIN FORT WALTON BEACH CONVENTION CENTER
EXTERIOR ACCESSIBILITY

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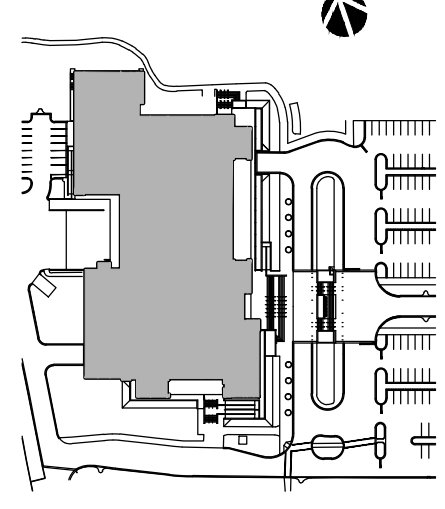
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~~230 4196~~
 230 4196



DAG ARCHITECTS
 DAG Architects, AAC000745
 1223 Airport Road, Destin, Florida 32541
 850.837.8152 www.dagarchitects.com

CONSTRUCTION DOCUMENT

DESTIN FORT WALTON BEACH CONVENTION CENTER
 EXTERIOR ACCESSIBILITY
 TASK ORDER No. 6B - C18-2638-TDD
 1250 MIRACLE STRIP PKWY SE
 FORT WALTON BEACH, FL 32548



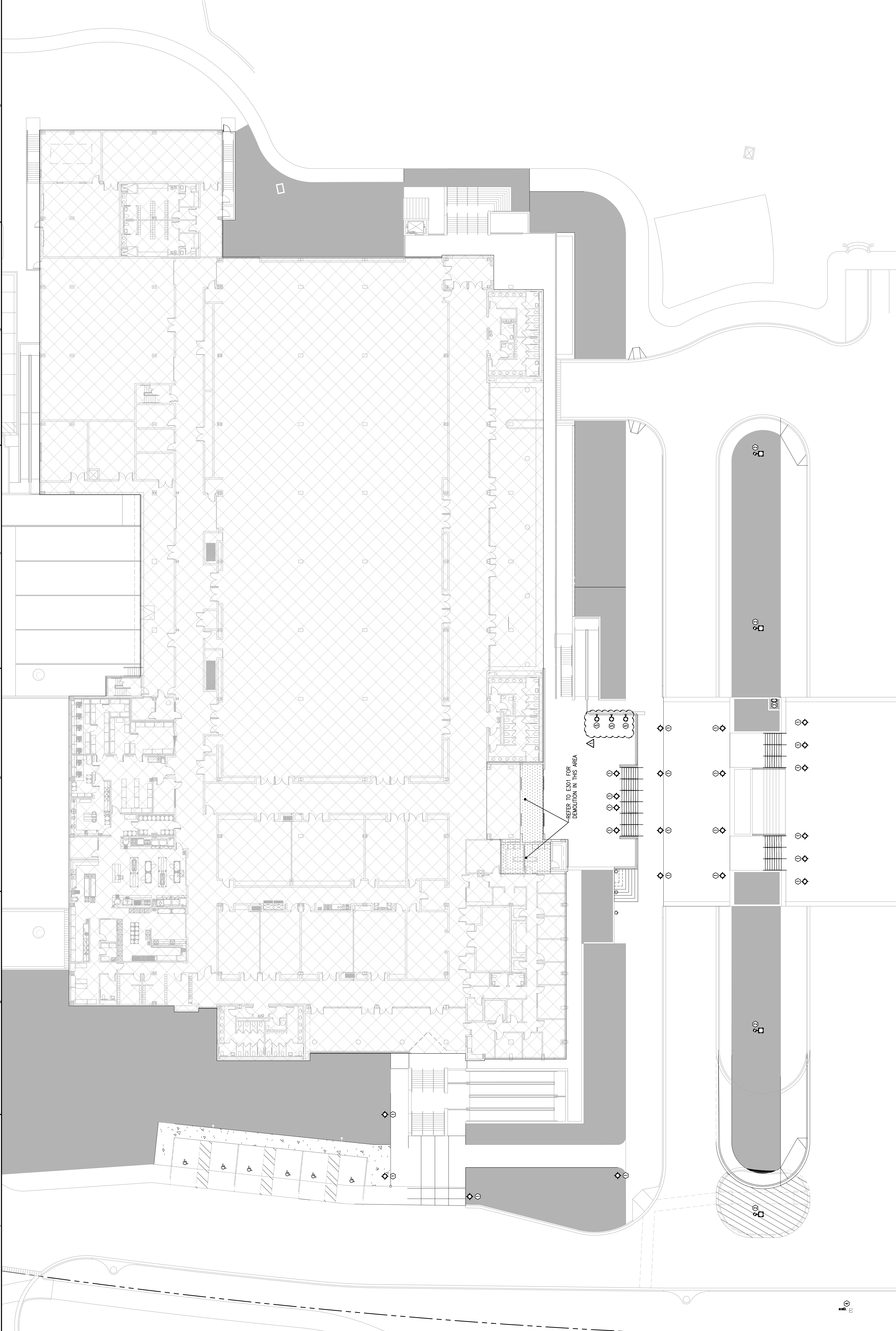
ITB TDD 71-20 ADDENDUM 1

No.	Description	Date
1	EXPOSURE REMOVAL	07/27/20

SITE PLAN - ELECTRICAL - DEMOLITION
 Project number: 170576
 Date: 06-12-2020
 PIC: PM

ED101

ITB TDD 71-20 ADDENDUM 1



- NOTES**
- ① REMOVE BOLLARD. REMOVE EXISTING CONDUCTORS AND CAP EXISTING CONDUIT. ADD PAVERS TO MATCH EXISTING.
 - ② REMOVE FIXTURE, POLE AND POLE BASE. REMOVE EXISTING CONDUCTORS AND CAP EXISTING CONDUIT.
 - ③ REMOVE EXISTING FOUNTAIN CONDUCTORS AND CAP EXISTING CONDUIT.
 - ④ REMOVE METER, PANEL AND EXISTING CONDUCTORS BACK TO EXISTING TRANSFORMER. REPAIR BACKBOARD AS REQUIRED.
 - ⑤ EXISTING CONDUIT. REMOVE EXISTING CONDUCTORS AND CAP EXISTING CONDUIT.

SITE PLAN - ELECTRICAL - DEMOLITION
 1/16" = 1'-0"
 1986
 I C E N G I N E E R S

142 Engineering Ave. SE
 Fort Lauderdale, Florida 33316
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ITB TDD 71-20 ADDENDUM 1
EXISTING ENTRY CONDUIT PHOTOS

